

# One-dimensional modeling of aquifer contamination using a meshless method

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## Abstract

**Background:** Water is one of the most important and fundamental needs in the lives of all living creatures. Owing to the recent drought around the world and reduction of surface water resources, groundwater resources are gaining more significance. Distribution and spread of contamination in groundwater resources make them unusable and thus aggravate the drought crisis in arid and semi-arid regions. Therefore, it is of utmost importance to protect groundwater resources from the input of pollutants and to reduce the amount of contaminants in them.

**Materials and Methods:** In this research, the transmission of pollution, identification of transmission and diffusion processes, and its modeling were studied in order to assist the practitioners in adopting appropriate strategies. Also, the Meshless Local Petrov-Galerkin method was used to solve the transmission equation of the porous medium, while moving the least squares was used as its approximation function. For verification purposes, the results of the numerical solution were compared with the exact solution. This comparison indicated the acceptable accuracy of the Meshless Local Petrov-Galerkin method for solving the transmission equation for porous media.

**Results:** The results of the numerical solution were compared with the calculated values of Agata's exact solution at different times for the one-dimensional equation of pollution in the porous medium. The calculated total error was 0.03, indicating that the Meshless Local Petrov-Galerkin method has an acceptable precision in simulating one-dimensional contamination in the aquifer.

**Conclusions:** This study showed that the numerical method of Meshless Local Petrov-Galerkin is suitable for modeling the transmission of pollutants in groundwater in one-dimensional mode and has acceptable accuracy.

**Keywords:** Contamination transmission equation, Ground water, Meshless local petrov-galerkin method, Moving least squares approximation function

## Introduction

Underground water, as the most important source of water supply in Iran, is used for drinking, agricultural, and industrial purposes. Therefore, its quality has a significant impact on the health of humans and the environment. The quality of groundwater is threatened by industrial activities, pesticides, underground reservoir leaks, oil and gas extraction, and landfill waste. Due to the recent drought and reduction of groundwater resources, the spread of pollution in these resources makes them unusable. Therefore, contamination of groundwater resources will exacerbate the drought crisis in the arid and semi-arid regions. Therefore, it is important to protect groundwater resources from contamination and reduce the amount of contaminants in them. In addition to the issue of

identifying pollutants and preventing their penetration, examining their transmission and distribution mechanisms to model the existing situation and predict future status is vital (1).

The best instrument for comprehending the behavior of pollution in a porous medium is the numerical solution of the governing equation. Efficient numerical models are used to predict the movement and transmission of pollutants in porous environments and for appropriate adjustment and management of the contaminated sites, as performing experiments on a real scale is time-consuming and costly. Conventional numerical methods for pollution transmission problems include finite difference methods, finite element, boundary element, and finite volume, the

selection of which depends on the type of solution and user's convenience. Comprehension of finite difference models compared to the finite elements is simpler, yet the application of this method is limited to the rectangular meshes, which are not relevant here. Finite element methods and finite volume are flexible in relation to the problem geometry. The finite element method is not suitable for transmission of contamination because the transmission operator is asymmetric and is not applicable in the finite element method.

Moreover, due to the node-to-node fluctuations, the finite element method is not suitable for transmission of the contamination. Mesh-based methods are flawed in problems with high transmission speeds and/or low spread (2). The aforementioned numerical methods require the computation field to be disjoint. Therefore, meshless methods are considered due to the lack of need for disjoint computational field and its consequent problems. The original idea of meshless methods was presented by Gingold and Monaghan in 1977, in which they used the hydrodynamic method of smooth particles (3). The main purpose of using meshless methods is to keep a distance from confining and approximating the entire field of the problem using only nodes. A large number of meshless methods for numerical integration of the weak form equations require a background mesh in the entire field domain (4). One of these meshless methods is the Meshless Local Petrov-Galerkin (MLPG) method, which was first introduced by Atluri and Zhu in 1998 (5). The whole process of solving using this method is independent of any mesh or background cell, and thus, it is an entirely meshless method (6).

In this paper, the MLPG method was used for the numerical solution of the transmission of pollution in a porous medium, in which the form function of moving least squares and the weak form of partial differential equations were used to construct a series of algebraic equations, the results of which were compared with the exact results.

## Materials and Methods

### Meshless Local Petrov-Galerkin (MLPG) numerical method

The MLPG method was first introduced by Atluri and Zhu in 1998. This method solves the equations based on the weak local form and the moving least squares. The main advantage of this method is that unlike the previous numerical methods, no meshing is required in the interpolation process (7). In this method, the local background mesh is used to solve the integral equations, and since the generation of a

local background mesh is much easier than generating the corresponding mesh for the entire domain of the problem, MLPG can be used as a truly meshless method, or at least close to an ideal meshless method (8).

### Moving Least Squares (MLS) approximation

For the first time in 1992, Nirvelles and colleagues presented the moving least squares approximation to generate form functions. Currently, MLS is used to generate the form functions for many meshless methods. The two main popular features of MLS are: (1) the approximate field function if the entire problem domain is soft and continuous, and (2) the ability to construct an approximation with arbitrary order of compatibility (8).

If  $U(X)$  is a field variation function in the given range  $\Omega$ , the approximation  $U^h(X)$  at point  $X$  is represented by  $U^h(X)$ . The MLS approximation first describes the field function in the form below:

$$U^h(X) = \sum_j^m P_j(X) a_j(X) = P^T(X) a(X) \quad (1)$$

Where  $m$  is the number of the constituent monomials of  $P(X)$ , while  $a(X)$  is the coefficient vector with the following form:

$$a^T(X) = \{a_1(X) a_2(X) a_3(X) \dots a_m(X)\} \quad (2)$$

In Equation (1),  $P(X)$  is a vector of base functions, which often contains the maximum number of the necessary monomials to obtain the minimum of completeness. In one-dimensional space, a polynomial base of degree  $m$  is as follows:

$$a^T(X) = \{1 \ x \ x^2 \ \dots \ x^m\} \quad (3)$$

The coefficients vector  $a(X)$  in (1) is determined using the values of the function in a set of nodes that are located in the support domain of the  $X$ -point. The number of nodes that are used locally to estimate the value of the function at point  $X$  is determined by the support domain.

It is noteworthy that  $\hat{U}(X)$  is an optional function of  $X$ . A residual weight function is generated using estimated values from the field function and node parameters, that is, (7):

$$J = \sum_i^n W(X - X_i) [U^h(X - X_i) - U(X_i)]^2 = \sum_i^n W(X - X_i) [P^T(X_i) a(X)] \quad (4)$$

In which  $W(X - X_i)$  is a function of the weight and  $X_i$  is the node parameter of the field changes in

node I.

A proper weight function should be non-zero in just a small vicinity of  $X_i$ , which is called the effect range of node I. One useful feature of the approximation of the moving least squares is that their continuity is equivalent to the continuity of their weight function. That is, by choosing a suitable weight function, approximations with a high degree of continuity can be found (9).

In this research, the cubic spline weight function is used (8):

$$W(x - x_i) = w_i(x) = \begin{cases} 2/3 - 4\bar{r}_i^2 + 4\bar{r}_i^3\bar{r}_i & 0.5 \leq \bar{r}_i \leq 1 \\ 4/3 - 4\bar{r}_i + 4\bar{r}_i^2 - 4/3\bar{r}_i^3 & 0 \leq \bar{r}_i \leq 0.5 \\ 0 & \bar{r}_i \geq 1 \end{cases} \quad (5)$$

$$\bar{r}_i = \frac{d_i}{r_w} = \frac{|x - x_i|}{r_w} \quad (6)$$

In (6),  $r_w$  is the effect radius of node  $x_i$ ,  $\bar{r}_i$ , it represents the normal distance and  $d_i$  is the distance from node  $X_i$  (10, 11)

**Shape function**

In the MLS approximation, the coefficients of  $a(X)$  at the arbitrary point  $X$  are chosen in such a way that the residual weight function shown in the preceding relation, namely  $J$ , is minimized, thus implying:

$$\frac{\partial J}{\partial a} = 0 \quad (7)$$

As a result:

$$a(X) = B(X)U_s A^{-1}(X) \quad (8)$$

In which,  $a(X)U_s$  and  $B(X)$  are calculated using the following equations:

$$A(X) = \sum_i^n W(X_i)p(X_i)P^T(X_i) \quad (9)$$

$$B(X) = [B_1 B_2 B_3 \dots B_n] \quad (10)$$

$$B(X) = [W_1 p(x_1) W_2 p(x_2) \dots W_n p(x_n)] \quad (11)$$

$$U_s = \{U_1 U_2 U_3 \dots U_n\}^T \quad (12)$$

By using (8) in Equation (1), the MLS approximation may be represented as follows:

$$U^h(X) = \sum_i^n \sum_j^m P_j(X)(A^{-1}(X)B(X))_{ji} U_i \quad (13)$$

$$U^h(X) = \sum_i^n \phi_i(X) U_i \quad (14)$$

In which  $U^h(X)$  is the function approximation,  $\phi_i(X)$  is the shape function and  $U_i$  is the node parameter. Thus, the MLS shape function is defined in the following form (8):

$$\phi_i(X) = \sum_j^m P_j(X)(A^{-1}(X)B(X))_{ji} = P^T A^{-1} B_i \quad (15)$$

**Governing equation**

The two main parameters in the transmission of pollution are (1) the displacement of contamination between the two points and (2) dispersion during displacement. Displacement is the movement of the pollutant along with the flow of fluid in the soil. Dispersion is the expansion of the contamination area during the flow of fluid within the system (12). Estimation of the contamination diffusion processes in the porous medium is carried out by the differential equation of transmission and distribution. This equation is presented with simplifying assumptions such as homogeneity, isotropy, and saturation of porous media as well as permanency of flow (13).

$$D_L \left( \frac{\partial^2 C}{\partial x^2} \right) - \bar{v}_x \frac{\partial C}{\partial x} = \frac{\partial C}{\partial t} \quad (16)$$

In the above equation (16),  $C$  is the concentration of pollutant,  $D_L$  is the diffusion coefficient in direction  $X$ ,  $\bar{v}_x$  and is the average velocity of pollution leakage. The formulation of its weak form using the residual weight method is as follows:

$$\int_{\Omega} \left[ D_L \left( \frac{\partial^2 C}{\partial x^2} \right) - \bar{v}_x \frac{\partial C}{\partial x} - \frac{\partial C}{\partial t} \right] W d\Omega = 0 \quad (17)$$

Using the partial integral and the divergence lemma, we arrive at:

$$\int_{\Omega} W \left[ D_L \left( \frac{\partial^2 C}{\partial x^2} \right) - V_x \frac{\partial C}{\partial x} - \frac{\partial C}{\partial t} \right] d\Omega = 0 \quad (18)$$

$$\int_{\Omega} W \left[ D_L \left( \frac{\partial^2 C}{\partial x^2} \right) - V_x \frac{\partial C}{\partial x} \right] d\Omega = \int_{\Omega} \left[ W \frac{\partial C}{\partial t} \right] d\Omega \quad (19)$$

$$C = \sum_{i=1}^n \phi_i C_i \tag{20}$$

$W$  is the weight function while  $\phi_i$  is the shape function, which are different in the MLPG method. Finally, by placing the equation (20) in (19), we have:

$$\int_{\Omega} (\frac{\partial w}{\partial x} D_L \frac{\partial \phi}{\partial x}) d\Omega c - \int_{\Gamma} (\frac{\partial w}{\partial x} V_X \phi) d\Gamma c = \int_{\Gamma} (W D_L \frac{\partial c}{\partial n}) d\Gamma + \int_{\Omega} (W \frac{\partial c}{\partial t}) d\Omega \tag{21}$$

The above relation can be summarized in the following matrix:

$$KU = F \tag{22}$$

In which  $K$  is the matrix of hardness,  $U$  is the matrix of the unknowns and  $F$  is the loading matrix. In this equation, the domain boundaries under study are of two types.

(A) the boundaries where the value of the function is known (Dirichlet boundary)

(B) the boundaries in which the first-order differential relative to the vector perpendicular to the boundary at the same point is known (Newman's boundary)

$$c = c_0 \Gamma_c \tag{23}$$

$$\frac{\partial c}{\partial n} = \bar{q} \Gamma_t \tag{24}$$

Eqs. (23) and (24) respectively represent the Dirichlet and Newman boundaries (14). The Newman boundary condition is directly applied, but since the MLS function does not satisfy the condition of the Kronecker delta, the Dirichlet boundary condition cannot be directly applied (8). In this study, the penalty method was used to apply this boundary condition.

**Numerical example**

The equation for the transmission of a pollutant in an assumed aquifer is studied in one dimension, as shown in Fig. 1. The contaminant source is considered as a continuous point. In this case, the flow is considered uniform and along the x-axis. The concentration of pollutant  $t=0$  in the aquifer is zero. The concentration of the pollutant in  $t>0$  the left border of the aquifer is  $C_0$ . The boundary conditions of this problem are as follows:

Initial conditions:

$$C(x, 0) = 0 \quad \text{For all } x \tag{25}$$

Boundary conditions:

$$C(0, t) = C_0 \quad x = 0 \tag{26}$$

$$C(\infty, t) = 0 \quad x > 0 \tag{27}$$

The diffusion coefficient in the horizontal direction is equal to  $D_L = 1 \text{ m}^2/\text{day}$ , and the leakage velocity is equal to  $V_X = 0.1 \text{ m}^2/\text{day}$ . The range is equal to  $L = 100 \text{ m}$ , and the contaminant concentration is equal to  $c_0 = 10 \text{ mg/l}$ . Boundary condition in this problem is a combination of Dirichlet and Newman boundaries. Points are distributed at a distance of 10 meters uniformly in the range of the problem. The assumed geometry of the aquifer and the boundary conditions of the problem are indicated in Fig 1.

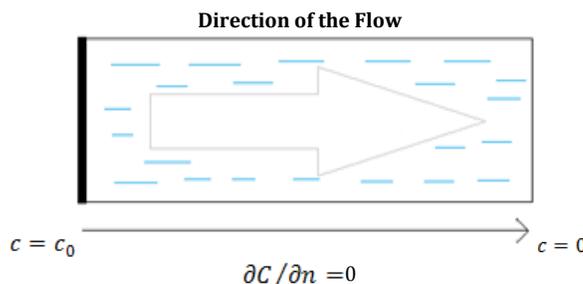


Fig 1. Domain in the one-dimensional mode

**Results**

Node values were approximated using the MLPG method at different time intervals. The values of the numerical solution were compared with the calculated values of Agata's exact solution at different times (15), the results of which are presented in Fig. 2 and Table 1. It can be concluded from this comparison that the MLPG method has acceptable accuracy.

**Estimation of total error**

In order to estimate the total error, a function is defined in the range of study, which is then quantitatively calculated and presented (16):

$$E = \frac{\sum_{i=1}^n |c_i^{exact} - c_i^{num}|}{\sum_{i=1}^n |c_i^{exact}|} \tag{28}$$

Where  $E$  is the average of the error of the function,  $c_i^{exact}$  is the exact value,  $c_i^{num}$  is the numerical value of the function, and  $n$  is the number of nodes. The overall estimated error is 0.03, thus indicating that the meshless method yields acceptable values.

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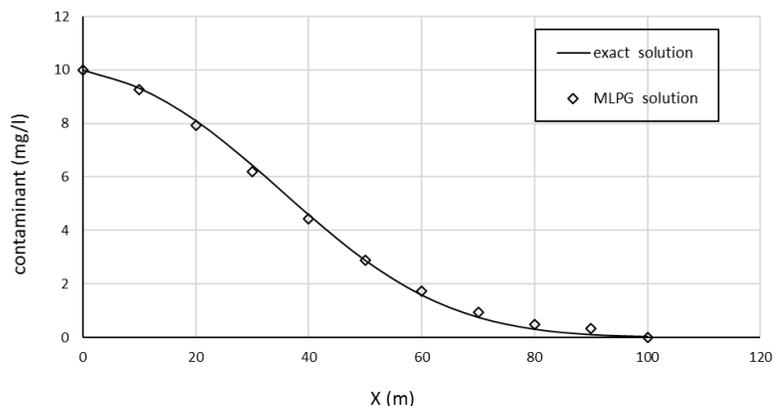


Fig 2. Comparison of numerical and exact values at time t = 300 day

Table 1. Results of the Petrov-Galerkin method in 300 days

Node number	Coordinates x (m)	Results of the MLPG method	Exact solution	Relative error of the MLPG method
1	0	10	10	0
2	10	9.2504	9.3216	0.0076
3	20	7.9279	8.1077	0.0221
4	30	6.2109	6.4367	0.0351
5	40	4.4296	4.5802	0.0329
6	50	2.8818	2.8805	0.0004
7	60	1.7192	1.5846	0.0850
8	70	0.9476	0.7567	0.2523
9	80	0.4948	0.3119	0.5862
10	90	0.3183	0.1106	1.8792
11	100	0	0.0335	1

### Discussion

Figure 2 shows MLPG method and exact solution in a graph. The correspondness of MLPG results to exact one indicates the high accuracy of MLPG in simulation of contamination transmission. This figure also confirms the boundary conditions of problem. In the first place as the boundary conditions is constant contamination (Dirichlet), it is equal to 10m in all time steps. Table 1 presents the value of contamination which is derived from MLPG in each node in comparison of their real value. The Mfree model shows more error in 10th node which its relative error equal to 1.87. Although in the rest of nodes the relative error is less than 1.

### Conclusion

In the present circumstances, the issue of groundwater pollution is more critical for the exploitation of the limited fresh water resources than ever. Modeling this phenomenon is essential in order to understand the status quo and to predict the future status of aquifers. The advantage of this method is the lack of need for meshing of the solution range, which reduces the time it takes to perform the calculations. In this study, using the MLPG method, the modeling of the transmission of pollutants in groundwater in homogeneous mode

was investigated. For this reason, the governing equations of the problem were discretized using the Petrov-Galerkin local method, and for approximation of the shape function, moving least squares was used. The results of this method were compared with the exact results. The results of this comparison indicate the acceptable accuracy of the Petrov-Galerkin local method.

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### Conflicts of Interest

The authors declare no conflicts of interest.

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